## South Dakota Division

of Trasportalion
Federal Highway Administration

116 East Dakota Avenue, Suite A
Pierre, South Dakota 57501
Phone: 605-224-8033
Fax: 605-224-8307
SouthDakota.FHWA@dot.gov

Mr. Steve Gramm, P.E.
Planning Engineer
In Reply Refer To: HDA-SD

South Dakota Department of Transportation
700 East Broadway Avenue
Pierre, SD 57501-3110
November 13, 2017

Subject: Interchange Modification Justification Report (IMJR); I-29, Exit 77 ( $41^{\text {st }}$ Street); IM 0293, PCN 03RA, Minnehaha County

Dear Mr. Gramm:
The Federal Highway Administration has completed our review of your November 7, 2017 submittal requesting approval of an access modification to Interstate 29 at Exit 77 ( $41^{\text {st }}$ Street) in Minnehaha County.

Based on an engineering and operational review of the justification report, we hereby find the proposed modifications acceptable. Final approval of the IMJR will be granted upon satisfactory completion of the environmental process and the determination that the selected alternative results in no major design changes. Once the environmental document is approved, Federal funds can then be used for final design, purchasing right-of-way, and/or construction activities for the project.

If you have any questions, please feel free to call Mark Hoines at 605-776-1010.
Sincerely,

R. Kirk Fredrichs

South Dakota Division Administrator

E-Copies: Mike Behm, SDDOT, Planning and Engineering Mark Leiferman, SDDOT, Project Development<br>Mark Hoines, FHWA<br>Mark Clausen, FHWA<br>Brett Hestdalen, FHWA



I-29 Exit 77 (41st ${ }^{\text {st }}$.)
Interchange Modification Justification Report

Project PL 0100(84) , PCN 05MH
Project IM 0293(106)77P, PCN 03RA
Sioux Falls, SD
October 19, 2017

# Interchange Modification Justification Report I-29 Exit 77 ( $41^{\text {st }} \mathrm{St}$.) 

Project PL 0100(84), PCN 05MH
Project IM 0293(106)77P, PCN 03RA

Sioux Falls, South Dakota
October 19, 2017

Prepared for:


South Dakota Department of Transportation Office of Project Development
700 East Broadway Avenue
Pierre, South Dakota 57501-2586

## Prepared by: <br> $1-32$

HDR Engineering, Inc. 6300 South Old Village Place

Suite 100
Sioux Falls, SD 57108

The South Dakota Department of Transportation provides services without regard to race, color, gender, religion, national origin, age or disability, according to the provisions contained in SDCL 20-13, Title VI of the Civil Rights Act of 1964, the Rehabilitation Act of 1973, as amended, the Americans With Disabilities Act of 1990 and Executive Order 12898, Federal Actions to Address Environmental Justice in Minority Populations and Low Income Populations, 1994. To request additional information on the SDDOT's Title VI/Nondiscrimination policy or to file a discrimination complaint, please contact the Department's Civil Rights Office at 605-773-3540.

The preparation of this report has been financed in part through grant(s) from the Federal Highway Administration and Federal Transit Administration, U.S. Department of Transportation, under the State Planning and Research Program, Section 505 or Metropolitan Planning Program Section 104(f) of Title 23, U.S. Code. The contents of this report do not necessarily reflect the official views or policy of the U.S. Department of Transportation.

## TABLE OF CONTENTS

EXECUTIVE SUMMARY ..... 1

1. INTRODUCTION ..... 3
1.1 Background ..... 3
1.2 Purpose ..... 3
1.3 Project Location ..... 3
2. METHODOLOGY ..... 6
3. EXISTING CONDITIONS ..... 7
3.1 Demographics ..... 7
3.2 Existing Land Use ..... 7
3.3 Existing Roadway Network ..... 7
3.4 Alternative Travel Modes ..... 8
3.5 Interchanges ..... 8
3.6 Existing Data ..... 8
3.7 Operational Performance ..... 8
3.8 Existing Safety Conditions ..... 14
3.9 Existing Environmental Constraints ..... 27
4. PROJECT NEED ..... 28
5. ALTERNATIVES ..... 29
6. FUTURE YEAR TRAFFIC ..... 32
7. ALTERNATIVES ANALYSIS ..... 66
7.1 Conformance with Transportation Plans ..... 66
7.2 Compliance with Policies and Engineering Standards ..... 66
7.3 Environmental Impacts ..... 69
7.4 Safety ..... 69
7.5 Operational Performance ..... 69
7.6 Evaluation of Alternatives ..... 69
7.7 Coordination ..... 69
8. FUNDING PLAN ..... 70
9. RECOMMENDATIONS ..... 71
APPENDIX
1 - Purpose and Need Memo
2 - Methods and Assumptions Document
3 - Technical Memo 1: Existing Traffic Conditions
4 - Technical Memo 2: Existing Conditions Crash Analysis
5 - Technical Memo 3: Future Traffic Conditions
6 - Technical Memo 4: Predictive Crash Analysis
7 - Technical Memo 5: Carolyn Avenue Alternative Analysis
8 - Technical Memo 6: Interchange Alternatives Evaluation
9 - Technical Memo 7: 41 ${ }^{\text {st }}$ St. West of I-29 Alternatives Evaluation
10 - Technical Memo 8: 11 - Concept Signing Plan
12 - Interchange Area Air Photos

## FIGURES

| Figure 1 - Study Area | 5 |
| :---: | :---: |
| Figure 2 - Existing Traffic Volumes and Signalized Intersection Level of Service (west of I-29) | 10 |
| Figure 3 - Existing Traffic Volumes and Signalized Intersection Level of Service (east of I-29) | 11 |
| Figure 4 - Existing Multimodal Peak Hour Level of Service | 12 |
| Figure 5 - Existing Peak Hour Balanced Traffic Volumes and I-29 Level of Service | 13 |
| Figure 6 - Signalized Intersection Crash Analysis Summary, Years 2010-2015(west of I-29) | 17 |
| Figure 7 - Signalized Intersection Crash Analysis Summary, Years 2010-2015 (east of I-29) | 18 |
| Figure 8 - Arterial Street Segment Crash Analysis Summary, Years 2010-2015 (west of I-29) | 20 |
| Figure 9 - Arterial Street Segment Crash Analysis Summary, Years 2010-2015 (east of I-29) | 21 |
| Figure 10 - Interstate Segments for Crash Analysis (see Table 3) | 23 |
| Figure 11 - Interstate Ramps for Crash Analysis (see Table 4) | 25 |
| Figure I-1 - Single Point Interchange Alternative | 30 |
| Figurel-2 - Diverging Diamond Interchange Alternative | 31 |
| Figure 12a - Summary of Signalized Intersections with Predicted Peak Hour Low LOS | 33 |
| Figure 12-2023 No-Build Traffic Volumes \& Signalized Intersection Level of Service (west) | 34 |
| Figure 13-2023 No-Build Traffic Volumes \& Signalized Intersection Level of Service (east) | 35 |
| Figure 14-2023 Local Improvements Only Traffic Volumes and Signalized LOS (west) | 36 |
| Figure 15-2023 Local Improvements Only Traffic Volumes and Signalized LOS (east) | 37 |
| Figure 16-2023 Single Point Traffic Volumes and Signalized LOS (west) | 38 |
| Figure 17-2023 Single Point Traffic Volumes and Signalized LOS (east) | 39 |
| Figure 18-2023 DDI Traffic Volumes and Signalized LOS (west) | 40 |
| Figure 19-2023 DDI Traffic Volumes and Signalized LOS (east) | 41 |
| Figure 20-2045 No-Build Traffic Volumes and Signalized LOS (west) | 42 |
| Figure 21 - 2045 No-Build Traffic Volumes and Signalized LOS (east) | 43 |
| Figure 22-2045 Local Improvements Only Traffic Volumes and Signalized LOS (west) | 44 |
| Figure 23-2045 Local Improvements Only Traffic Volumes and Signalized LOS (east) | 45 |
| Figure 24-2045 Single Point Traffic Volumes and Signalized LOS (west) | 46 |
| Figure 25-2045 Single Point Traffic Volumes and Signalized LOS (east) | 47 |
| Figure 26 - 2045 DDI Traffic Volumes and Signalized LOS (west) | 48 |
| Figure 27 - 2045 DDI Traffic Volumes and Signalized LOS (east) | 49 |
| Figure 28-2023 No-Build Multimodal Peak Hour Level of Service | 50 |
| Figure 29-2023 Local Improvements Only Multimodal Peak Hour Level of Service | 51 |
| Figure 30-2023 Single Point Multimodal Peak Hour Level of Service | 52 |
| Figure 31-2023 DDI Multimodal Peak Hour Level of Service | 53 |
| Figure 32-2045 No-Build Multimodal Peak Hour Level of Service | 54 |
| Figure 33-2045 Local Improvements Only Multimodal Peak Hour Level of Service | 55 |
| Figure 34-2045 Single Point Multimodal Peak Hour Level of Service | 56 |
| Figure 35-2045 DDI Multimodal Peak Hour Level of Service | 57 |
| Figure 36-2023 No-Build Peak Hour Balanced Traffic Volumes and I-29 Level of Service | 58 |
| Figure 37-2023 Local Improvements Only Peak Hour Balanced Traffic Volumes and I-29 LOS | 59 |
| Figure 38-2023 Single Point Peak Hour Balanced Traffic Volumes and I-29 LOS | 60 |
| Figure 39-2023 DDI Peak Hour Balanced Traffic Volumes and I-29 LOS | 61 |
| Figure $40-2045$ No-Build Peak Hour Balanced Traffic Volumes and I-29 LOS | 62 |
| Figure 41-2045 Local Improvements Only Peak Hour Balanced Traffic Volumes and I-29 LOS | 63 |
| Figure 42-2045 Single Point Peak Hour Balanced Traffic Volumes and I-29 LOS | 64 |
| Figure 43-2045 DDI Peak Hour Balanced Traffic Volumes and I-29 LOS | 65 |
| Figure 44 - Year 2023 and 2045 Traffic Volumes and Critical Non-Signalized Intersection LOS | 67 |
| Figure 45 - Year 2023 and 2045 Traffic Volumes and Critical Non-Signalized Intersection LOS | 68 |
| Figure 46 - Diverging Diamond Interchange Layout with Control of Access and ROW | 74 |

## TABLES

Table 1 - Signalized Intersection Crash Rates (2010-2015)
Table 2 - Arterial Segments Crash Rates (2010-2015) 22
Table 3 - Interstate Segment Crash Rates (2010-2015) 24
Table 4 - Interstate Ramp Crash Rates (2010-2015) 26
Table 5 - Anticipated Funding Allocation Breakdown 70

## Abbreviations

| AASHTO | American Association of State Highway Transportation Officials |
| :--- | :--- |
| CATEX | Categorical Exclusion |
| CMF | Crash Modification Factor |
| CRM | Critical Rate Method |
| FHWA | Federal Highway Administration |
| HCS | Highway Capacity Software |
| HOT | High Occupancy Transit |
| HOV | High Occupancy Vehicle |
| HSM | Highway Safety Manual |
| IMJR | Interchange Modification Justification Report |
| LOS | Level of Service |
| MRM | Mileage Reference Marker |
| MPO | Metropolitan Planning Organization |
| MSA | Metropolitan Statistical Area |
| SDDOT | South Dakota Department of Transportation |
| STIP | Statewide Transportation Improvement Program |
| TAZ | Traffic Analysis Zone |
| TIP | Transportation Improvement Program |

## EXECUTIVE SUMMARY

This Interchange Modification Justification Report (IMJR) report provides technical analysis related to proposed changes to the existing $41^{\text {st }}$ Street interchange (Exit 77) on Interstate 29 in Sioux Falls, SD.

The proposed action is a reconfiguration of the existing $41^{\text {st }}$ Street interchange on Interstate 29 in Sioux Falls, SD. The action is proposed to bring the existing interchange up to current design standards and provide appropriate operational capacity for future traffic demand. No adverse impacts to the Interstate highway system are forecast due to the proposed change.

The Federal policy considerations and requirements have been addressed in the Recommendations section of this report and summary responses to the eight requirements are provided below.

The proposed change is a reconfiguration of an existing interchange and improvements to the existing crossroad facility. The changes will correct current interchange and crossroad design deficiencies including short access spacing to an adjacent intersection, low peak hour ramp terminal level of service, and low peak hour crossroad level of service. The proposed change does not result in any new access points on the Interstate Highway System.

The concept alternatives involve changes to the geometric design of an existing interchange and changes to the crossroad arterial street to satisfy current design standards and meet the transportation needs in the study area. Mass transit reaches a limited market in South Dakota and HOV facilities are currently not in use because they have not been shown to be economically feasible. Neither mass transit nor HOV facilities will correct design deficiencies or provide sufficient relief to future travel demand within the study planning horizon.

The operational and safety analysis contained in this study shows that the proposed build alternatives are not expected to adversely affect the safety or efficiency of the Interstate system. The build alternatives are also expected to improve access management on the crossroad in the vicinity of the interchange.

The conceptual signing plan for the diverging diamond interchange alternative is shown in Appendix 11.

The proposed access is a reconfiguration of an existing interchange with full access to an arterial city street and includes all movements. The conceptual drawings have been prepared using current standards and further design using current standards is anticipated and will receive additional reviews throughout the next steps of the design.

The proposal is the result of land use and transportation plans prepared within the MPO process, including the Sioux Falls MPO Long Range Transportation Plan. The 20172020 STIP includes this Project in the Developmental STIP portion in the 2021-2023 funding and construction years.

Analysis techniques included evaluation of operational capacity using Highway Capacity Manual 2010 techniques via HCS 2010. Highway Safety Manual techniques were used to the extent possible in this report. Other techniques and reference materials are detailed in a Methods and Assumptions document prepared for this study and signed by City of Sioux Falls, South Dakota Department of Transportation, and Federal Highway Administration participants in July, 2014 and modified as necessary throughout the study. The Methods and Assumptions document is included in the Appendix 2.

## 1. INTRODUCTION

### 1.1 Background

SDDOT, the City of Sioux Falls, the Sioux Falls Metropolitan Planning Organization, and the Federal Highway Administration have conducted a study to evaluate the design, operations, policy and funding implications of modifying the $41^{\text {st }}$ Street interchange (Exit 77) on I-29 in Sioux Falls, SD. The existing interchange serves an urban arterial corridor that has carried the largest traffic volumes in the state of South Dakota, including a significant amount of commuting traffic. This Interchange Modification Justification Report is being prepared in conjunction with applicable environmental reviews and analyses, and will provide traffic analysis for the selection of a preferred alternative.

### 1.2 Purpose

The 2010 Decennial Interstate Corridor Study identified the I-29 Exit 77 interchange as a top priority project needing near-term improvement. The study cited safety concerns, geometric deficiencies at the interchange, and low peak hour level of service and recommended a DDI interchange as the most feasible interchange improvement alternative.

The I-29 Exit 77 (41 ${ }^{\text {st }}$ Street) Crossroad Corridor Study (2012) analyzed the interchange area and the crossroad arterial street in more detail and recommended a package of arterial street improvements and conversion of the interchange to a DDI configuration. The study again cited safety concerns, geometric deficiencies and level of service in determining the need for interchange improvements.

This study continues the previous planning work and provides the necessary analysis for consideration by SDDOT and FHWA.

### 1.3 Project Location

The subject interchange is at mileage reference marker 77 on Interstate 29, in southwest Sioux Falls, SD. This location is within the Sioux Falls MPO and also within the developed Sioux Falls urban area. The adjacent interchanges on I-29 are $26^{\text {th }}$ Street (Exit 78) and the I-229 system interchange (Exit 75). Interchange spacing is approximately .85 miles to $26^{\text {th }}$ Street from the subject interchange and about 1.78 miles to Interstate 229.

There are several local roadways in the vicinity of the interchange. The crossroad, $41^{\text {st }}$ Street, is a five-lane urban street west of the interchange and a seven-lane urban street east of the interchange. $41^{\text {st }}$ Street is intersected by Marion Road approximately $1 / 2$ mile west of I-29. Two collector roadways, Valley View Drive/Holbrook Avenue and Terry Avenue intersect 41st Street approximately 1 mile and $1 / 4$ mile west of I-29, respectively. Madelyn Lane/Gateway Boulevard and Meadow Avenue are the closest intersecting local streets west of I-29 at 800' and 400', respectively from the I-29 southbound ramp terminal. The nearest intersection on $41^{\text {st }}$ Street east of I-29 is Carolyn Avenue, approximately 230 ' east of the I-29 northbound ramp terminal. Empire Place, Shirley Avenue and Louise Avenue intersect $41^{\text {st }}$ Street approximately 0.18 mile, 0.29 , and 0.46 miles east of I-29, respectively. All these intersecting streets are controlled by traffic signals except the $41^{\text {st }} /$ Carolyn intersection, which is controlled by a stop sign on the

Carolyn Avenue approach. Improvements within the study area, including projects undertaken by the City of Sioux Falls, are planned to provide additional capacity along $41^{\text {st }}$ Street. A solution to congestion issues in the interchange area, however, may require modification of the interchange itself.

Other local streets intersect I-29 at the adjacent $26^{\text {th }}$ Street interchange or lie within the street network in the vicinity of the subject interchange. The study area, therefore, has been defined as Interstate 29, from MRM 75 to MRM 79, including Exits 75, 77, and 78, 41 ${ }^{\text {st }}$ Street from Valley View Road/Holbrook Avenue to just west of Louise Avenue, and portions of 26th that are part of the local street network served by the subject interchange and the adjoining interchanges. The study area is shown in Figure 1.

### 1.4 Logical Termini

- West limits - Marion Road was determined to be the westerly logical termini for the interchange study. This is mainly due to the need to widen $41^{\text {st }}$ Street to 3 through lanes in each direction from I-29 to Marion Road to accommodate future traffic volumes. Widening of the Marion Road/41 ${ }^{\text {st }}$ Street intersection is necessary to add turn lanes for adequate intersection capacity.
- East limits - The City intends to expand the Shirley Avenue/41 ${ }^{\text {st }}$ Street intersection and realign the extension of Shirley Avenue south of $41^{\text {st }}$ Street. This will become the main entrance and exit to the Empire Mall from $41^{\text {st }}$ Street. The relocation of Sioux Falls Ford dealership along the south side of $41^{\text {st }}$ Street west of Shirley Avenue allows for widening of $41^{\text {st }}$ Street and adding raised medians and a right turn lane. Property is not feasibly available east of Shirley Avenue for expansion of $41^{\text {st }}$ Street. Therefore, Shirley Avenue is a reasonable east logical terminus for this project.
- $41^{\text {st }}$ Street/Louise Avenue intersection - Although existing and future LOS at this intersection is below acceptable criteria for peak traffic periods, existing developments adjacent to the intersection limit the feasible opportunities for improvement of the intersection. Therefore, this intersection is not within in the logical termini for the I-29 Exit $77\left(41^{\text {st }}\right.$ Street) interchange study. Traffic impacts at the $41^{\text {st }}$ Street/Louise Avenue intersection resulting from capacity improvements at the I-29 Exit 77 (41st Street) interchange and along 41st Street have been closely evaluated in the traffic assessment. Potential improvement options for the $41^{\text {st }}$ Street/Louise Avenue intersection are included in a separate, subsequent Louise Avenue corridor project between 34th Street and 49th Street.
- Marion Road limits - Marion Road has been upgraded to 2 through lanes in each direction from $39^{\text {th }}$ Street to the north. Therefore, the north logical terminus for Marion Road begins at $39^{\text {th }}$ Street. Widening of Marin Road extends through the $41^{\text {st }}$ Street intersection so that 2 through lanes and dual left turn lanes are provided for needed capacity. The south logical terminus for Marion Road is the distance needed to transition to the existing 3 lane section. Allowance is made for the potential future expansion of Marion Road to a 5 lane section south of 41 st Street.
For further background on the logical termini for the project, see the Purpose and Need Memo included in Appendix 1.



## 2. METHODOLOGY

Preparation of this report included the following work tasks:

- Data gathering
- Review previous Interstate studies and coordinate with preparation of the environmental studies, including feasible alternatives and the recommended alternative.
- Determine existing and future operational characteristics of Interstate and local street facilities.
- Prepare a deliverable report.

Traffic forecasts were prepared using output from the regional travel demand model maintained by the City of Sioux Falls and the Sioux Falls MPO. Traffic operations were analyzed using the Highway Capacity Manual techniques using HCS 2010 software modules and Highway Safety Manual techniques.

This IMJR document is organized in accordance with section 2.5.2 of FHWA's Interstate System Access Information Guide, August 2010.

Additional details on methodology can be found in the Methods and Assumptions agreement prepared for this project and included in the Appendix 2.

## 3. EXISTING CONDITIONS

### 3.1 Demographics

The Sioux Falls metropolitan area enjoys a robust economy and sustained population growth. During the period 1980 - 2000 the population grew at a steady rate of between $2 \%$ and $3 \%$ per year. Even in the face of the recent recession, the population continued to grow at an annual rate of $1 \%-2 \%$ per year and the 2010 Census shows the city with a population of 153,888 , while the Metropolitan Statistical Area (MSA) had a population of 228,261 and the market area had a population of $1,043,450$. (Market area is a term used in economics and human geography describing the area surrounding a central place, from which people are attracted to use the place's goods or services.)

Generally, employment for the Sioux Falls area has grown at approximately the same rate as the population and unemployment is currently near 2\% in Sioux Falls, compared with a statewide rate of $2.7 \%$, regional rate of $3.0 \%$, and a national unemployment rate of $5.0 \%$.

### 3.2 Existing Land Use

The study area is comprised mainly of commercial uses ranging from big-box retail to light industrial with a smaller presence of single family and multifamily residential uses. The study area Traffic Analysis Zones (TAZ's) currently reflect the existing population and employment inputs. The future-year TAZ's show gradual infill of uses similar to those currently existing in the study area.

The future land use plan for Sioux Falls shows continued development of the existing land uses for this portion of the urban area.

### 3.3 Existing Roadway Network

As previously identified, the existing major roadways within the study area include:

- Interstate 29 - expanded to three lanes in each direction during this study, with auxiliary lanes between interchanges north of $41^{\text {st }}$ Street
- $41^{\text {st }}$ Street - 5-lane urban arterial roadway west of I-29 that transitions to a 7-lane urban roadway east of I-29
- $26^{\text {th }}$ Street - 4-lane divided urban arterial roadway
- Marion Road - 5-lane urban arterial roadway north of $41^{\text {st }}$ Street and 3-lane urban arterial south of $41^{\text {st }}$ Street
- Louise Avenue - 5-lane urban arterial roadway
- Valley View Drive/Holbrook Avenue - 2-lane urban collector roadway
- Terry Avenue - 2-lane urban collector roadway
- Lorraine Drive - 4-lane divided urban collector roadway
- Carolyn Avenue - 2-lane urban local roadway
- West Empire Place - 4-lane divided private roadway
- Shirley Avenue - 3-lane urban collector roadway
- $34^{\text {th }}$ Street -2 -lane urban collector roadway
- $38^{\text {th }}$ Street -2 -lane urban local roadway


### 3.4 Alternative Travel Modes

Travel within the study area is primarily by automobile. Pedestrian activity is relatively high along the $41^{\text {st }}$ Street corridor with City counts showing a peak range of 55 to 128 pedestrians per day at various locations for the years from 2003 to 2010. Pedestrian counts were also taken in March 2016 in conjunction with the traffic counts. Due to the time of year, the number of pedestrians was not as high as the previous peak levels from the City's 2003 to 2010 data. However, there were still a significant number of pedestrians along the corridor. One reason for the relatively high pedestrian use within the corridor is that the area west of I-29 contains multiple disadvantaged, handicapped, and assisted living facilities which generate pedestrians going to the Empire Mall and other commercial areas. The regional recreational trail system runs along the Skunk Creek/Big Sioux River channel on the north and east boundaries of the study area. The area is currently served by municipal transit routes 10 and 11, operating on portions of $41^{\text {st }}$ Street, Marion Road, Shirley Avenue, and Louise Avenue. Buses operate on 30 minute headways during peak demand.

### 3.5 Interchanges

Interchanges within the study area include:

- I-29/I-229 (Exit 75) - this system interchange is currently a trumpet configuration.
- I-29/41 ${ }^{\text {st }}$ Street (Exit 77) - the subject interchange is a diamond configuration. Both ramp termini are controlled by traffic signals.
- $\mathrm{I}-29 / 26^{\text {th }}$ Street (Exit 78) - this service interchange is currently a diamond configuration with both ramp termini controlled by traffic signals.

Aerial photos of the existing interchanges have been included in the Appendix 12.

### 3.6 Existing Data

New peak hour weekday and Saturday turning movement counts were gathered from March 9 to 13,2016 for study area analysis intersections. Additional arterial roadway study data were available from the participating agencies, including traffic counts, crash data, and raw travel demand model output. The available data was supplemented with Interstate system counts, vehicle classifications, and traffic observations. The data is recent and of high quality.

### 3.7 Operational Performance

Operation performance of highways is evaluated in terms of the quality of service, which describes how well a transportation facility operates from the traveler's perspective. Quality of service is usually measured with "Level of Service", a letter grade similar to those used in school. Level of service A refers to uncongested traffic conditions, with levels of service B through E describing increasingly more congested conditions and level of service F describing the highest congestion or saturation. Level of service is determined in different ways for different roadway facilities, with Interstate highway facilities evaluated in terms of vehicle density, urban intersections evaluated in terms of vehicle delay, and other facilities evaluated using other measures of roadway dynamics. All quality of service is determined using techniques developed for the Highway Capacity Manual (HCM), published by the Transportation Research Board.

The existing study area roadways were evaluated using the HCM methodologies for Interstate highways and urban streets. SDDOT has established a minimum level of service standard of "C" for interstate facilities, including ramp terminal intersections. The City of Sioux Falls has established a minimum level of service standard of "D" for arterial signalized intersections. Interstate 29 operates at acceptable levels of service under existing conditions. The arterial street system, however, experiences peak hour congestion at the following locations:

- $26^{\text {th }}$ Street/Marion Rd. (AM peak)
- $26^{\text {th }}$ Street/l-29 SB (Saturday peak)
- $26^{\text {th }}$ Street/l-29 NB (AM peak)
- $41^{\text {st }}$ Street/Louise Ave. (AM, PM, and Saturday peaks)

Certain movements experienced low levels of service or queues that exceeded the length of the available storage during particular peak hours. The eastbound left turn at $41^{\text {st }}$ Street/l- 29 NB is an example of this characteristic, with the left turn queue extending through the $41^{\text {st }}$ Street $/ \mathrm{l}-29$ SB intersection during the AM peak.

Multimodal level of service varies widely throughout the $26^{\text {th }}$ Street and $41^{\text {st }}$ Street corridors. The lowest levels of service are related to locations with the absence of specific facilities for pedestrians and bicyclists in these corridors.

There are several stop-controlled intersections in the study area that are anticipated to play a role in future improvement alternatives. Those intersections are listed below along with peak hour stopped-approach level of service information:

- $38^{\text {th }}$ Street/Carolyn Avenue: currently an uncontrolled intersection - no level of service analysis available.
- $38^{\text {th }}$ Street/Shirley Avenue: AM-B, PM-D, SAT-D
- $41^{\text {st }}$ Street/Carolyn Avenue: AM-C, PM-F, SAT-F

Daily traffic volumes in the study area are shown in Figures 2 and 3. Peak hour traffic volumes and levels of service are summarized in Figures 2 through 5. Supporting analysis printouts are provided in the Appendix 3.





### 3.8 Existing Safety Conditions

An analysis of existing safety conditions was conducted, based on crash records provided by SDDOT. The analysis was conducted using the Critical Rate Method, as described in the Highway Safety Manual (HSM), published by the American Association of State Highway and Transportation Officials (AASHTO).

## Highway Safety Manual definition:

- Critical Rate Method (CRM) - a method in which the observed crash rate at each site is compared to a calculated critical crash rate that is unique to each site.

Crash data for the years 2010 through 2015 were provided by SDDOT and reviewed to identify any existing crash concentrations/crash trends and develop potential crash mitigation measures. Analyses were conducted for the following roadway facilities:

- Arterial street intersections
- Arterial street segments
- Interstate mainline segments
- Interstate ramp segments

Critical crash rates were calculated for each segment or intersection and used to identify portions of the study area that displayed crash rates higher than the critical rate. Each of the above-critical locations is discussed in subsequent sections of this memorandum.

## Segment and Intersection Crash Rates:

The study area was divided into segments representing:

- Arterial street intersections (Figures 6, 7, Table 1)
- Arterial street segments (Figures 8, 9, Table 2)
- Interstate mainline segments (Figure 10, Table 3)
- Interstate ramp segments (Figure 11, Table 4)

Mainline and ramp sections were each analyzed separately to allow calculation of representative crash rates and critical rates for each type of Interstate feature. Rate calculations included other Interstate highway sections in the Sioux Falls urban area, resulting in more robust statistical representation of crash rates.

The arterial street intersections named in the M\&A document were grouped for calculation of crash rates and critical rates.

Details of the crash records for each segment and intersection are contained in the Appendix 4.

## Segment and Intersection Critical Crash Rates:

Critical crash rates were calculated based on the statistical populations in each spreadsheet, using the methods shown in the Highway Safety Manual (American Association of State Highway and Transportation Officials (AASHTO), 2010). Those segments and intersections that lay outside the critical rates are shown in red coloration in the last column of each spreadsheet. All the segments and intersections that lay within the critical rate are shown in green.

## Crash Trends:

Review of the crash summaries for each Interstate and arterial street section revealed a few crash trends:

- Slightly elevated incidence of single vehicle crashes on the Interstate during inclement weather.
- Rear-end crashes at arterial intersections with the highest levels of congestion.
- Angle crashes at driveways in the arterial street segments near $41^{\text {st }}$ Street/Louise Ave.
- Rear-end crashes on $41^{\text {st }}$ Street in the arterial street segments bounding the Interstate 29 interchange.


## Potential Mitigation Measures:

The general crash trends identified above suggest several potential strategies for reducing crash rates within the study area:

- Consider ITS-related means of communicating slippery roadway conditions to drivers and continue aggressive winter maintenance.
- Addition of high-friction surface courses on bridges, curves and weaving areas may help reduce crashes that occur during inclement weather. A section of high-friction surfacing has been installed on I-229 and may provide useful guidance about employing this technique for safety enhancement.
- Additional access management measures on $41^{\text {st }}$ Street and Louise Avenue, particularly near the $41^{\text {st } / L o u i s e ~ i n t e r s e c t i o n . ~}$
- Reconfiguration of the $41^{\text {st }}$ Street interchange on I-29 to reduce $41^{\text {st }}$ Street queues and enhance $41^{\text {st }}$ Street traffic flow.

Each of the arterial intersections and Interstate segments that were identified as having crash rates outside of the critical rates are discussed in the following points.

## Arterial Intersections:

- $41^{\text {st }}$ Street/Marion Road $-53 \%$ of the crashes at this intersection involved rear-end incidents, while $37 \%$ involved angle incidents. Peak hour queues and congestion may be leading to increased crash rates.
- $41^{\text {st }}$ Street/l-29 SB $-78 \%$ of the crashes at this intersection involved rear-end incidents, while $17 \%$ involved angle incidents. Peak hour queues and congestion may be leading to increased crash rates.
- $41^{\text {st }}$ Street/l-29 NB $-53 \%$ of the crashes at this intersection involved rear-end incidents, while $40 \%$ involved angle incidents. Peak hour queues and congestion may be leading to increased crash rates.
- $41^{\text {st }}$ Street/Shirley Avenue - 63\% of the crashes at this intersection involved angle incidents, while 31\% involved rear-end incidents. The intersection configuration and offset of Shirley Avenue may be leading to increased crash rates. Peak hour queues and congestion may also play a role in crash incidence.
- $41^{\text {st }}$ Street/Louise Avenue - $54 \%$ of the crashes at this intersection involved rear-end incidents, while 37\% involved angle incidents. Peak hour queues and congestion may be leading to increased crash rates.


## Arterial Street Segments:

- Louise Avenue - Wal-Mart driveway to $41^{\text {st }}$ Street - angle crashes at driveways appear to be leading to higher-than-normal crash rates. Queues from the adjacent intersections frequently back up into this segment making driveway maneuvers difficult.
- $41^{\text {st }}$ Street - Terry Avenue to I-29 SB - peak hour queues from the I-29 interchange result in a high rate of rear-end crashes.
- $41^{\text {st }}$ Street - I-29 NB to West Empire Place - peak hour queues from the I-29 interchange result in a high rate of rear-end crashes.
- $41^{\text {st }}$ Street - Shirley Avenue to Louise Avenue - angle crashes at driveways and rearend crashes appear to be leading to higher-than-normal crash rates. Queues from the adjacent intersections frequently back up into this segment making driveway maneuvers difficult and creating unexpected encounters with the back of the queue.


## Interstate Segments:

- I-29 NB, $41^{\text {st }}$ Street to $26^{\text {th }}$ Street - single vehicle crashes with fixed objects appear to be leading to an elevated crash rate in this segment.
- I-29 NB, $26^{\text {th }}$ Street off ramp to $26^{\text {th }}$ Street on ramp - single vehicle crashes with fixed objects during slippery roadway conditions represent the predominant crash type.


## Interstate Ramps:

No critical Interstate ramp segments were identified.



TABLE 1 - SIGNALIZED INTERSECTION CRASH RATES (2010-2015) I-29/41ST ST. IMJR

| INTERSECTION | NUMBER CRASHES | DAILY VOLUME | MEV ${ }^{1}$ | CRASH <br> RATE | TEV* ${ }^{2}$ | CRITICAL RATE | DIFFERENCE |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2: 26TH/LORRAINE | 12 | 24400 | 44.53 | 0.27 | 6575.34 | 0.93 | -0.66 |
| 3: 26TH/I-29 SB | 42 | 27500 | 50.19 | 0.84 | 23013.70 | 0.92 | -0.08 |
| 4: 26TH/I-29 NB | 33 | 31200 | 56.94 | 0.58 | 18082.19 | 0.91 | -0.33 |
| 5: 26TH/SHIRLEY | 36 | 30500 | 55.66 | 0.65 | 19726.03 | 0.91 | -0.26 |
| 9: LOUISE/WAL-MART | 4 | 31100 | 56.76 | 0.07 | 2191.78 | 0.91 | -0.84 |
| 11: 41ST/MARION | 77 | 40500 | 73.91 | 1.04 | 42191.78 | 0.88 | 0.16 |
| 12: 41ST/TERRY | 45 | 32300 | 58.95 | 0.76 | 24657.53 | 0.90 | -0.14 |
| 13: 41ST/I-29 SB | 91 | 38400 | 70.08 | 1.30 | 49863.01 | 0.89 | 0.41 |
| 14: 41ST/I-29 NB | 95 | 37500 | 68.44 | 1.39 | 52054.79 | 0.89 | 0.50 |
| 16: 41ST/W. EMPIRE | 56 | 38300 | 69.90 | 0.80 | 30684.93 | 0.89 | -0.09 |
| 17: 41ST/SHIRLEY | 67 | 36100 | 65.88 | 1.02 | 36712.33 | 0.89 | 0.12 |
| 18: 41ST/LOUISE | 101 | 61700 | 112.60 | 0.90 | 55342.47 | 0.85 | 0.05 |
| COLUMN TOTAL | 1676220 |  |  | 1196027.40 |  |  |  |
| WEIGHTED AVERAGE RATE |  |  |  | 0.71 |  |  |  |

${ }^{1} \mathrm{MEV}=\mathrm{MILLION}$ ENTERING VEHICLES
${ }^{2}$ TEV*R $=$ TOTAL ENTERING VEHICLES TIMES CRASH RATE
SOURCE: HIGHWAY SAFETY MANUAL, FIRST EDITION, 2010, AASHTO
NOTE: CRITICAL RATE STATISTICS ARE BASED ON A LARGE SAMPLE OF SIGNALIZED ARTERIAL INTERSECTIONS IN SIOUX FALLS NOTE: THE RED AND GREEN HIGHLIGHTS PROVIDE A QUICK VISUAL INDICATION OF WHICH INTERSECTIONS HAVE A CRASH RATE OUTSIDE OF THE STATISTICAL CRITICAL RATE. THE RED INTERSECTIONS ARE CONSIDERED OUTSIDE THE CRITICAL IMPROVEMENTS), WHILE THE GREEN SEGMENTS ARE INSIDE THE CRITICAL RATE. THIS CRASH ANALYSIS IS BEING PERFORMED AS PART OF THE IMJR PROCESS, A PLANNING PROCESS FOR FUTURE POTENTIAL INTERCHANGE PROJECTS. THE HIGHWAY SAFETY MANUAL (HSM) IS BEING USED AS A REFERENCE FOR PROPER PLANNING OF THE ROADWAY FEATURES THAT MAY BE ASSOCIATED WITH POTENTIAL INTERCHANGE IMPROVEMENTS. PROVIDING A COMPLETE EXPLANATION OF THE CRITICAL RATE METHOD WOULD REQUIRE RESTATEMENT OF A LARGE PORTION OF THE



TABLE 2 - ARTERIAL SEGMENT CRASH RATES (2010-2015)
I-29/41ST ST. IMJR

${ }^{1} \mathrm{MVMT}=\mathrm{MILLION}$ VEHICLE MILES TRAVELED
${ }^{2}$ TEV*R $=$ TOTAL ENTERING VEHICLES PER DAY, TIMES OBSERVED CRASH RATE
SOURCE: HIGHWAY SAFETY MANUAL, FIRST EDITION, 2010, AASHTO
NOTE: CRITICAL RATE STATISTICS ARE BASED ON A LARGE SAMPLE OF ARTERIAL SEGMENTS IN SIOUX FALLS
NOTE: THE RED AND GREEN HIGHLIGHTS PROVIDE A QUICK VISUAL INDICATION OF WHICH SEGMENTS HAVE A CRASH RATE OUTSIDE OF THE STATISTICAL POSSIBLY REQUIRING ATTENTION AND SAFETY IMPROVEMENTS) WHILE THE GREEN SEGMENTS ARE INSIDE THE CRITICAL RATE. THE CRASH ANALYSIS IS BEING PERFORMED AS PART OF THE IMJR PROCESS, A PLANNING PROCESS FOR FUTURE POTENTIAL INTERCHANGE PROJECTS. THE HIGHWAY SAFETY MANUAL IS BEING USED AS A REFERENCE FOR PROPER PLANNING OF THE ROADWAY FEATURES THAT MAY BE ASSOCIATED WITH POTENTIAL INTERCHANGE IMPROVEMENTS. PROVIDING A COMPLETE EXPLANATION OF THE CRITICAL RATE METHOD WOULD REQUIRE RESTATEMENT OF LARGE PORTIONS OF THE HIGHWAY SAFETY MANUAL IN THIS DOCUMENT. INSTEAD, THE HSM IS INCLUDED BY REFERENCE.


TABLE 3 - INTERSTATE SEGMENT CRASH RATES (2010-2015)
I-29/41ST ST. IMJR

${ }^{1} \mathrm{MVMT}=\mathrm{MILLION}$ VEHICLE MILES TRAVELED
${ }^{2}$ TEV*R $=$ TOTAL ENTERING VEHICLES PER DAY, TIMES OBSERVED CRASH RATE
SOURCE: HIGHWAY SAFETY MANUAL, FIRST EDITION, 2010, AASHTO
NOTE: CRITICAL RATE STATISTICS ARE BASED ON A LARGE SAMPLE OF INTERSTATE SEGMENTS IN SIOUX FALLS
NOTE: THE RED AND GREEN HIGHLIGHTS PROVIDE A QUICK VISUAL INDICATION OF WHICH SEGMENTS HAVE A CRASH RATE OUTSIDE OF THE STATISTICAL POSSIBLY REQUIRING ATTENTION AND SAFETY IMPROVEMENTS) WHILE THE GREEN SEGMENTS ARE INSIDE THE CRITICAL RATE. THE CRASH ANALYSIS IS being performed as part of the imjr process, a planning process for future potential interchange projects. the highway safety MANUAL IS BEING USED AS A REFERENCE FOR PROPER PLANNING OF THE ROADWAY FEATURES THAT MAY BE ASSOCIATED WITH POTENTIAL INTERCHANGE IMPROVEMENTS. PROVIDING A COMPLETE EXPLANATION OF THE CRITICAL RATE METHOD WOULD REQUIRE RESTATEMENT OF LARGE PORTIONS OF THE highway safety manual in this document. Instead, the him is included by reference.


TABLE 4 - INTERSTATE RAMP CRASH RATES (2010-2015)
I-29/41ST ST. IMJR

| TRAVEL DIRECTION | SEGMENT | NUMBER CRASHES | SEGMENT <br> LENGTH | DAILY VOLUME | MVMT ${ }^{1}$ | CRASH RATE | TEV* $R^{2}$ | CRITICAL RATE | DIFFERENCE |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| SB | 1-26TH STREET OFF RAMP | 5 | 0.188 | 12700 | 4.36 | 1.15 | 11067.72 | 3.13 | -1.99 |
| SB | $2-26$ TH STREET ON RAMP | 1 | 0.265 | 3400 | 1.64 | 0.61 | 5590.71 | 4.01 | -3.40 |
| SB | 3-41ST STREET OFF RAMP | 7 | 0.27 | 13800 | 6.80 | 1.03 | 13405.62 | 2.87 | -1.84 |
| SB | 4-41ST STREET ON RAMP | 2 | 0.186 | 5100 | 1.73 | 1.16 | 4414.55 | 3.95 | -2.79 |
| SB | 5 - I-229 OFF RAMP | 12 | 0.979 | 8700 | 15.54 | 0.77 | 11269.45 | 2.54 | -1.76 |
| SB | 6-1-229 ON RAMP | 19 | 1.26 | 9200 | 21.16 | 0.90 | 10243.67 | 2.44 | -1.55 |
| NB | 7-1-229 OFF RAMP | 4 | 0.554 | 6800 | 6.88 | 0.58 | 11687.74 | 2.87 | -2.29 |
| NB | 8-1-229 ON RAMP | 5 | 0.376 | 8800 | 6.04 | 0.83 | 10627.87 | 2.94 | -2.11 |
| NB | 9-41ST STREET OFF RAMP | 2 | 0.185 | 5700 | 1.92 | 1.04 | 5484.72 | 3.83 | -2.79 |
| NB | 10-41ST STREET ON RAMP | 3 | 0.339 | 8900 | 5.51 | 0.54 | 16335.08 | 2.99 | -2.44 |
| NB | 11 - 26TH STREET OFF RAMP | 1 | 0.209 | 4400 | 1.68 | 0.60 | 7384.39 | 3.98 | -3.39 |
| NB | 12-26TH STREET ON RAMP | 2 | 0.265 | 10100 | 4.88 | 0.41 | 24667.29 | 3.06 | -2.65 |
|  | COLUMN TOTAL | 195590 |  |  |  | 1.93 376535.73 |  |  |  |
|  | WEIGHTED AVERAGE RATE |  |  |  |  |  |  |  |  |

${ }^{1} \mathrm{MVMT}=\mathrm{MILLION}$ VEHICLE MILES TRAVELED
${ }^{2}$ TEV*R $=$ TOTAL ENTERING VEHICLES TIMES CRASH RATE
SOURCE: HIGHWAY SAFETY MANUAL, FIRST EDITION, 2010, AASHTO
NOTE: CRITICAL RATE STATISTICS ARE BASED ON A LARGE SAMPLE OF INTERSTATE RAMPS IN SIOUX FALLS
NOTE: THE RED AND GREEN HIGHLIGHTS PROVIDE A QUICK VISUAL INDICATION OF WHICH SEGMENTS HAVE A CRASH RATE OUTSIDE OF THE STATISTICAL CRITICAL RATE. THE RED SEGMENTS ARE CONSIDERED OUTSIDE THE CRITICAL RATE (HAVING A CRASH RATE OUTSIDE THE NORMAL RANGE AND POSSIBLY REQUIRING ATTENTION AND SAFETY IMPROVEMENTS) WHILE THE GREEN SEGMENTS ARE INSIDE THE CRITICAL RATE. THE CRASH ANALYSIS IS being performed as part of the imjr process, a planning process for future potential interchange projects. the highway safety MANUAL IS BEING USED AS A REFERENCE FOR PROPER PLANNING OF THE ROADWAY FEATURES THAT MAY BE ASSOCIATED WITH POTENTIAL INTERCHANGE IMPROVEMENTS. PROVIDING A COMPLETE EXPLANATION OF THE CRITICAL RATE METHOD WOULD REQUIRE RESTATEMENT OF LARGE PORTIONS OF THE highway safety manual in this document. Instead, the him is included by reference.

### 3.9 Existing Environmental Constraints

The environmental studies and analyses completed during this Interchange Modification Justification Report preparation were used to refine, screen and eliminate alternatives based, in part, on environmental impacts identified.

## 4. PROJECT NEED

The environmental evaluation process has formulated a purpose and need statement for this project, included in Appendix 1. Previous studies, including the 2010 Decennial Interstate Corridor Study and the I-29 Exit 77 ( $41^{\text {st }}$ Street) Crossroad Corridor Study, have identified the need to improve the Exit 77 interchange to address safety concerns, correct geometric deficiencies, and improve low peak hour levels of service.

## 5. ALTERNATIVES

Previous studies have identified improvements on the $41^{\text {st }}$ Street crossroad to provide adequate roadway capacity and improve safety. The following improvement concepts were developed to address the interchange area needs:

- No-Build Concept Option
- Local Improvements Only (previously identified improvements on the $41^{\text {st }}$ Street crossroad, no change to interchange configuration)
- Single point interchange (includes previously identified crossroad improvements)
- Diverging diamond interchange (DDI) (includes previously identified crossroad improvements)
The Single-Point and Diverging Diamond interchange concepts are shown in the following pages. The Concept Options were evaluated in a Technical Memorandum. The memo, including the concept options, is included in Appendix 8.




## 6. FUTURE YEAR TRAFFIC

Traffic forecasts for the study area were prepared using the regional travel demand model maintained by the City of Sioux Falls and the Sioux Falls MPO. The model horizon year is 2045 and is based on local land use plans. Forecasts for the proposed 2023 construction completion year were also produced. Future year traffic demand reflects planned improvements. Peak hour traffic volumes and levels of service for the four scenarios are shown in Figures 12 through 43. Level of service output sheets from HCS 2010 are included in the Appendix 5.

The 2023 operational analysis revealed the following findings:

- Many intersections experienced peak hour conditions where the queue storage ratio of at least one movement exceeds 1.0 - defined as level of service $F^{*}$
- The $41^{\text {st }}$ St./Marion Rd. intersection experiences level of service E during the PM peak hour under no-build conditions.
- The $41^{\text {st }}$ St./I-29 NB ramp terminal experiences level of service $F$ during the AM peak hour under the local improvements only alternative.
- Both interchange alternatives provide good level of service at AM, PM and Saturday peak hours.
- All Interstate mainline sections, weaving sections and ramps operate at level of service C or better.

The 2045 operational analysis revealed the following findings:

- Many intersections experienced peak hour conditions where the queue storage ratio of at least one movement exceeds 1.0 - defined as level of service $\mathrm{F}^{*}$
- The $41^{\text {st }}$ St./Marion Rd. intersection experiences level of service F during the AM and PM peak hours under no-build conditions.
- The $41^{\text {st }}$ St./Terry Ave. intersection experiences level of service E during the PM peak hour under no-build conditions.
- The $41^{\text {st }}$ St.II-29 NB ramp terminal experiences level of service E during the PM peak hour under no-build conditions and level of service F during the PM peak hour with the local improvements only alternative.
- Both interchange alternatives provide good level of service at AM, PM and Saturday peak hours.
- All Interstate mainline sections, weaving sections and ramps operate at level of service C or better.



































## 7. ALTERNATIVES ANALYSIS

The interchange improvement scenarios were analyzed and compared to determine which may be most suitable for meeting the project need. The areas of analysis and comparison are discussed in the following sections.

### 7.1 Conformance with Transportation Plans

Local (MPO and City) and State transportation plans, including the Sioux Falls MPO Long Range Transportation Plan have identified a need for an improved interchange at l-29/41 ${ }^{\text {st }}$ Street that meets design standards, improves safety, and provides adequate capacity to serve future travel demand. Both the single point and DDI alternatives satisfy the existing transportation planning considerations.

### 7.2 Compliance with Policies and Engineering Standards

Each of the interchange build options has used the latest guidance from AASHTO and FHWA and final design of any of the options may be accomplished without conflict with geometric design standards.

Access management was examined at adjacent local street intersections and commercial driveways. SDDOT design standards call for access spacing of at least 100' from the Interstate highway right-of-way line at ramp termini when rebuilding an existing urban interchange, but further recommend extending the control of access to meet the access spacing requirements established by South Dakota Administrative Rule 70:09. The Administrative Rules call for unsignalized access spacing of $100^{\prime}$ to 660' and minimum signalized access spacing of 1320', depending on the classification of the arterial street ( $41^{\text {st }}$ Street is not within SDDOT jurisdiction and is not currently classified in the State system). City of Sioux Falls design standards call for $1 / 4$ mile full access spacing on arterial roadways like $41^{\text {st }}$ Street, but list spacing of unsignalized partial access as "varies". Other guidelines and research recommends signalized intersections no closer than $1 / 4$ mile from interchange ramp termini, but allow unsignalized partial access at spacing less than $1 / 4$ mile. Several alternatives were examined for improvements on $41^{\text {st }}$ Street as part of the I-29 Exit 77 ( $41^{\text {st }}$ St.) Crossroad Corridor Study. Those alternatives each addressed retrofitting the accesses along $41^{\text {st }}$ Street to improve access management. That study recommended alternatives that include a center median on $41^{\text {st }}$ Street and the closure of the $41^{\text {st }}$ Street/Carolyn Avenue intersection.

The $41^{\text {st }}$ Street/Carolyn Avenue intersection was further evaluated in a Technical Memorandum as part of this study. That evaluation determined that the only feasible alternative for the intersection is to close access to Carolyn Avenue at $41^{\text {st }}$ Street because of its proximity to the I 29 interchange. Carolyn Avenue would be provided with a cul-de-sac for local traffic and local traffic would also be rerouted via $38^{\text {th }}$ Street and Shirley Avenue to reach $41^{\text {st }}$ Street. The analysis is summarized in Figures 44 and 45 and in the Technical Memorandum, reproduced in the Appendix 5.



### 7.3 Environmental Impacts

The proposed interchange scenarios have been thoroughly considered in the environmental analysis process, and a preferred alternative will be selected based on the study findings. Reconfiguration of the Exit 77 interchange to a single-point or DDI interchange may require small additional pieces of right of way along $41^{\text {st }}$ Street. Sound walls currently exist along I-29 within the study area and sound impacts of interchange reconfiguration will be studied as part of the environmental documentation.

### 7.4 Safety

A predictive analysis of the alternatives was conducted and the technical memo documenting that analysis is included in the Appendix 6. The predictive analysis indicates that both the single-point and DDI interchange configurations will provide better safety than the existing diamond configuration. The DDI alternative also shows lower crash frequency when compared to the single-point alternative. Therefore, the DDI alternative is predicted to provide the best safety performance of the alternatives considered.

### 7.5 Operational Performance

The operations of the alternative scenarios were evaluated using appropriate level of service techniques. Performance was analyzed for forecast traffic conditions with each of the alternatives. Results of the operational analyses are shown in Figures 12 through 43.

The analysis indicates that the single-point and DDI alternatives result in acceptable level of service on the Interstate system and the arterial street system. The no-build and local improvements only alternatives show sub-standard levels of service at the interchange ramp terminals.

### 7.6 Evaluation of Alternatives

A Technical Memorandum was prepared which compares the Single-Point and DDI interchange alternatives. That comparison determined that the DDI alternative provides the best technical solution for the transportation needs at the I-29 exit 77 interchange. A preferred alternative will be determined through the environmental process. The Technical Memorandum is reproduced in the Appendix 8.

### 7.7 Coordination

The $41^{\text {st }}$ Street corridor, including its interchange at Interstate 29, has been the subject of agency coordination and public involvement as part of the corridor planning process, including public meetings. The potential corridor improvements have also been the subject of review and coordination through the regular meetings of the MPO committees.

## 8. FUNDING PLAN

The 2017-2020 Statewide Transportation Improvements Program (STIP) does not contain a project for reconstruction of the $\mathrm{I}-29 / 41^{\text {st }}$ Street interchange. The interchange reconstruction project is in the SDDOT's developmental program between 2021 and 2024. Current SDDOT budget estimates for interchange improvements are shown below.

| Table 5 - Anticipated Funding Allocation Breakdown |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Project <br> Number | State <br> Funding <br> Category | Federal <br> Funding <br> Category | Federal <br> Funds <br> (\$ million) | State <br> Funds <br> (\$ million) | City <br> Funds <br> (\$ million) | Total <br> Funds <br> (\$ million) |
| IM <br> 0293(106)77 P <br> PCN03RA | Interstate | National <br> Highway <br> Performance | $\$ 17.6$ | $\$ 2.4$ |  | $\$ 20.0$ |
| P1400(16) P <br> PCN 05NF | Local <br> Urban <br> Systems | Surface <br> Transportation <br> Block Grant | $\$ 5.2$ |  | $\$ 13.7$ | $\$ 18.9$ |
|  | Total | $\$ 22.8$ | $\$ 2.4$ | $\$ 13.7$ | $\$ 38.9$ |  |

Note: As funding is fluid, category breakdown may be different at time of project authorization.
As the project is anticipated to be let to contract in Federal fiscal year 2023 or beyond, the inflated cost for the overall project is $\$ 43.4$ million.

## 9. RECOMMENDATIONS

The technical analysis contained in this Interchange Modification Justification Report has found that the diverging diamond interchange alternative provides the best technical solution for transportation needs in the study area. The single point interchange alternative also provides operations and safety improvements over the planned improvements to the local street system. The preferred option is being selected as part of the completion of the environmental review process.

The eight considerations and requirements for Interstate access are addressed below:

1) The need being addressed by the request cannot be adequately satisfied by existing interchanges to the Interstate, and/or local roads and streets in the corridor can neither provide the desired access, nor can they be reasonably improved (such as access control along surface streets, improving traffic control, modifying ramp terminals and intersections, adding turn bays or lengthening storage) to satisfactorily accommodate the design-year traffic demands.
The proposed change is a reconfiguration of an existing interchange and improvements to the existing crossroad facility. The changes will correct current interchange and crossroad design deficiencies. Based on the analysis of various alternatives, improvements on the crossroad ( $41^{\text {st }}$ Street) alone and/or modifications to the existing interchange configuration will not improve traffic conditions adequately to meet applicable criteria. Improvements to $41^{\text {st }}$ Street (such as intersection improvements east of I-29 and added through lanes west of I-29) are being planned by the City of Sioux Falls. The proposed change does not result in any new access points on the Interstate Highway System.
2) The need being addressed by the request cannot be adequately satisfied by reasonable transportation system management (such as ramp metering, mass transit, and HOV facilities), geometric design, and alternative improvements to the Interstate with the proposed change(s) in access.
The concept alternatives involve changes to the geometric design of an existing interchange and changes to the crossroad arterial street to satisfy current design standards and meet the transportation needs in the study area. Mass transit reaches a limited market in South Dakota and HOV facilities are currently not in use because they have not been shown to be economically feasible. Neither mass transit or HOV facilities will correct design deficiencies or provide sufficient relief to future travel demand within the study planning horizon.
3) An operational and safety analysis has concluded that the proposed change in access does not have a significant adverse impact on the safety and operation of the Interstate facility (which includes mainline lanes, existing, new, or modified ramps, ramp intersections with crossroad) or on the local street network based on both the current and the planned future traffic projections. The analysis shall, particularly in urbanized areas, include at least the first adjacent existing or proposed interchange on either side of the proposed change in access, shall be included in this analysis to the extent necessary to fully evaluate the safety and operational impacts that the proposed change
in access and other transportation improvements may have on the local street network. Requests for a proposed change in access must include a description and assessment of the impacts and ability of the proposed changes to safely and efficiently collect, distribute and accommodate traffic on the Interstate facility, ramps, intersection of ramps with crossroad, and local street network. Each request must also include a conceptual plan of the type and location of the signs proposed to support each design alternative.
The operational and safety analysis contained in this study shows that the proposed build scenarios are not expected to adversely affect the safety or efficiency of the Interstate system. All build alternatives are also expected to improve access management on the crossroad in the vicinity of the interchange.

The conceptual signing plan for the DDI alternative is shown in the Appendix 11.
Figure 46 illustrates these features of the Diverging Diamond Interchange:

- existing and proposed control of access limits
- existing and proposed right-of-way limits
- distances to adjacent access points

4) The proposed access connects to a public road only and will provide for all traffic movements. Less than "full interchanges" may be considered on a case-by-case basis for applications requiring special access for managed lanes (e.g., transit, HOVs, HOT lanes) or park and ride lots. The proposed access will be designed to meet or exceed current standards.

The proposed access is a reconfiguration of an existing interchange with full access to an arterial city street and includes all movements. The conceptual drawings have been prepared using current standards and further design using current standards is anticipated. Additional refinement will take place during the environmental and design phases of the project.
5) The proposal considers and is consistent with local and regional land use and transportation plans. Prior to receiving final approval, all requests for new or revised access must be included in an adopted Metropolitan Transportation Plan, in the adopted Statewide or Metropolitan Transportation Improvement Program (STIP or TIP), and the Congestion Management Process within transportation management areas, as appropriate, and as specified.
The proposal is the result of land use and transportation plans prepared within the MPO process, including the Sioux Falls MPO Long Range Transportation Plan. The 20172020 STIP includes this Project in the Developmental STIP portion in the 2021-2013 funding and construction years.
6) In corridors where the potential exists for future multiple interchange additions, a comprehensive corridor or network study must accompany all requests for new or revised access with recommendations that address all of the proposed and desired access changes within the context of a longer-range system or network plan.
SDDOT has prepared a study of the Interstate Highway System in South Dakota and this IMJR to address Interstate access needs. No additional interchanges are anticipated within the study area.
7) When a new or revised access point is due to a new, expanded, or substantial change in current or planned future development or land use, requests must demonstrate appropriate coordination has occurred between the development and any proposed transportation system improvements. The request must describe the commitments agreed upon to assure adequate collection and dispersion of the traffic resulting from the development with the adjoining local street network and Interstate access point.

The revised access point is due to geometric deficiencies in the existing interchange and general regional growth. The proposed change is not related to a particular development or land use change.
8) The proposal can be expected to be included as an alternative in the required environmental evaluation, review and processing. The proposal should include supporting information and current status of environmental processing.
The alternatives developed in this IMJR will serve as the technical basis for the environmental review process. The environmental studies are expected to be completed in time to allow construction in the 2021 to 2023 timeframe.


## APPENDIX

$$
\begin{gathered}
1 \text { - Purpose and Need Memo } \\
2 \text { - Methods and Assumptions Document } \\
3 \text { - Technical Memo 1: Existing Traffic Conditions } \\
4 \text { - Technical Memo 2: Existing Conditions Crash Analysis } \\
5 \text { - Technical Memo 3: Future Traffic Conditions } \\
6 \text { - Technical Memo 4: Predictive Crash Analysis } \\
7 \text { - Technical Memo 5: Carolyn Avenue Alternatives Evaluation } \\
8 \text { - Technical Memo 6: Interchange Alternatives Evaluation } \\
9 \text { - Technical Memo 7: 41 st St. West of I-29 Alternatives Evaluation } \\
10 \text { - Technical Memo 8: 41 }{ }^{\text {st }} \text { St. East of I-29 Alternatives Evaluation } \\
\text { 11 - Concept Signing Plan } \\
12 \text { - Interchange Area Air Photos }
\end{gathered}
$$

